

826 Park Street Castle Oaks Church

Preliminary Utility Report

December 4, 2025

Prepared for:

Wellspring Community
826 Park St #200
Castle Rock, CO 80109

Town of Castle Rock
SDP25-0023

Prepared by:



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Matrix Project No. 25.1727.001

Engineer's Statement:

The enclosed utility report, and exhibits, were prepared by me or under my direct supervision and is correct to the best of my knowledge and belief. Said utility report has been prepared in accordance with applicable Town of Castle Rock criteria and is in conformance with the master utility plans for the affected area. I accept responsibility for any liability caused by negligent acts, errors, or omissions on my part in preparing this report.

Signature: _____

April Lamar, PE

State Of Colorado No. 66406

Owner/Developer's Statement:

As Owner/Developer of Land identified within this report; I agree to proceed, implement and comply with all recommendations and requirements outlined herein.

Name of Developer

Authorized Signature

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Introduction

The Castle Oaks Church is located northeast of the intersection of Eighth St and Park St. It is bounded to the east by vacant commercial property and to the north by commercial development. The project scope is to add an asphalt parking lot to the vacant portion of the property on the east side, as well as minor pedestrian & sidewalk improvements. There will be no internal improvements to the existing building, and the site zoning and land use will not change. There are three access points to the property – one from the north and two from the south. All utilities will remain in place.

This Preliminary Utility Report has been prepared to demonstrate that there will be no change in daily demands for either sanitary and water services, therefore an in-depth water & wastewater system analysis is not required.

General Location and Description

More specifically, the project is located in a portion of the southwest $\frac{1}{4}$ of Section 2, Township 8 South, Range 67 West of the 6th Principal Meridian, Town of Castle Rock, County of Douglas, State of Colorado.

The existing building is addressed 826 Park St, which will serve as the project's proposed name. The site was recently rezoned to PD. As stated above, all site improvements are external to the building. All existing utilities are to remain in place and will continue to serve the existing building as is.



Figure 1 – Vicinity Map

Existing Water System

The building is currently served by an existing 1" water service tap and meter – which is combined with irrigation – and a 4" fire protection service line. Both service connection points and the meter are located at the northwest corner of the building and connect to a 6" water main line within Park Street. There are two existing hydrants – one directly NE of the intersection of Eighth St and Park Street, and the other at the northeast corner of the site. The building is currently sprinklered and the proposed site will remain sprinklered.

The water system will remain within the Town of Castle Rock's yellow pressure zone with a maximum HGL elevation of 6352, top elevation served at 6230 and low elevation served at 6065. The site is contained with a maximum elevation of 6178 and a minimum elevation of 6175. No additional land dedication is required.

Existing Water Demands

The existing church is two story and approximately 13,660 square feet. Since there are no proposed changes in zoning and changes to internal building layout, water demand and storage capacity will remain the same. The below water demand calculations are based on the Town of Castle Rock Water System Design 2018 Criteria Manual (CRWCM). The appendix includes additional excerpts from section 4.4.1 of the CRWCM for the equations used to get Average Daily Demand, Max Daily Demand, and Peak Hour Demand values. The existing 1" water meter is assigned 1.67 SFE.

$$\begin{aligned}\text{Church Typical ADD} &= 600 \text{ gpd/church} \\ \text{Max Daily Demand} &= \text{ADD} \times 2.5 = 600 \text{ gpd} \times 2.5 = 1,500 \text{ gpd} \\ \text{Peak Hour Demand} &= \text{ADD} \times 5.5 = 600 \text{ gpd} \times 5.5 = 3,300 \text{ gpd}\end{aligned}$$

Storage Requirements

Storage requirements are provided below based on Section 4.4.2 of the Town of Castle Rock Water Criteria. A submittal has been made to the Town of Castle Rock Fire and Rescue Department. Based on the existing building square footage and usage, an assumed 1,500 gpm for a duration of two hours has been used until an official fire flow has been assigned. The appendix contains an excerpt from the International Building Code's Fire Flow Calculation Table B105.1(2).

$$\begin{aligned}1,500 \text{ gpd} &= 1,500 \text{ Gallons} \\ + 1,500 \text{ gpm} &= 180,000 \text{ Gallons} \\ \text{Total Required Storage} &= 181,500 \text{ Gallons (0.182 MG)}\end{aligned}$$

Proposed Water System Analysis and Modeling

Per coordination with Town of Castle Rock, based on the proposed development and the existing utility conditions remaining as they exist today, water analysis and modeling are not required with this report. The water meter will remain in the same current location at the northwest corner of the building.

Existing Wastewater System

The existing building is currently served off from a 4" sewer service which connects to an 8" sanitary main line within the surface parking lot directly east of the church. There is an existing grease trap on the south side of the building which drains to the northeast and combines with the 4" sewer service and eventually into the existing 8" sewer main.

Existing Wastewater Flows

The existing church is two story and approximately 13,660 square feet. Since there are no proposed changes in zoning or square footage, wastewater demand will remain the same. The below wastewater demands based on the Town of Castle Rock Wastewater Criteria Manual (CRWWCM). The appendix includes excerpts from section 4.4.1.1 of the CRWWCM for the equations used to get Average Daily Flow, Peaking Factor, and Peak Daily Flow values.

$$\text{ADF} = 300 \text{ gpd/church} = 0.000300 \text{ MGD}$$

Peaking Factor & Peak design Flow Rate

Using the above Average Daily Flow of the proposed improvements building use, the Peaking Factor and furthermore the Peak Design Flow Rate can be calculated using the equations below. Note a maximum Peaking Factor of 5.0 shall be used.

$$\begin{aligned}\text{PF} &= 3.65 \times [(\text{ADF})^{-0.168}] \\ &= 3.65 \times [(0.000300 \text{ MGD})^{-0.168}] = \mathbf{14.2}\end{aligned}$$

$$\begin{aligned}\text{PDF} &= (\text{ADF} \times \text{PF}) + (0.1 \times \text{ADF}) \\ &= (300 \text{ gpd} \times 5.0) + (0.1 \times 300 \text{ gpd}) = \mathbf{1,530 \text{ gpd}}\end{aligned}$$

Proposed Wastewater System Analysis and Modeling

Per coordination with Town of Castle Rock, based on the existing utility conditions remaining as they are today, wastewater analysis and modeling are not required with this report.

Conclusion

Since there are no proposed changes in zoning or square footage of the existing church, water and wastewater demands and storage capacity will remain unchanged. Any proposed development of the existing water and wastewater infrastructure needed to serve this title is the responsibility of the developer and/or owner.

Appendix/Attachments

1. Vicinity Map
2. Preliminary Utility Plan
3. Castle Rock Water Demand Tables
4. Castle Rock Wastewater Demand Tables



VICINITY MAP
N.T.S.

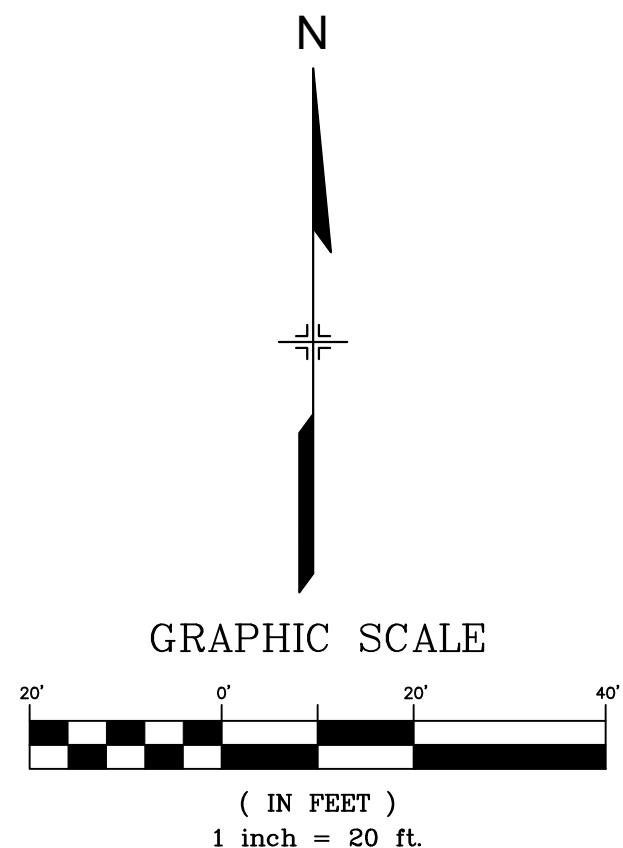
UT01

SHEET 6 OF 10

SITE DEVELOPMENT PLAN
LOT 2 HH SUBDIVISION 2ND AMENDMENT
TOWN OF CASTLE ROCK PROJECT NO. SDP25-0023



- | | | | |
|---|-------------------------------------|---|------------------------|
|  | EX. WATER MAIN | | |
|  | EX. SANITARY SEWER MAIN | | |
|  | EX. UNDERGROUND ELECTRIC LINE | | |
|  | EX. UNDERGROUND COMMUNICATIONS LINE | | |
|  | EX. UNDERGROUND GAS LINE | | |
|  | EX. STORM LINE | | |
|  | EX. FIRE HYDRANT |  | EX. STORM MANHOLE |
|  | EX. SANITARY MANHOLE |  | EX. ELECTRIC VAULT |
|  | EX. CLEAN OUT |  | EX. LIGHT POLE |
|  | EX. GAS METER |  | EX. TELEPHONE PEDESTAL |
|  | EX. AIR CONDITIONING UNIT | | |



1. THIS SITE IS LOCATED WITHIN THE TOWN OF CASTLE ROCK WATER PRESSURE ZONE 2 (YELLOW).
2. ALL PROPOSED EASEMENTS MUST BE RECORDED PRIOR TO ISSUANCE OF CONSTRUCTION PERMITS.
3. ALL UTILITY EASEMENTS FOR WATER AND WASTEWATER LINES SHALL BE FOR THE EXCLUSIVE USE OF THE TOWN SO THAT ACCESS AND MAINTENANCE MAY BE PERFORMED.
4. THE MINIMUM SEPARATION BETWEEN WATERLINES, SANITARY SEWER AND STORM SEWER LINES IS 10 FEET.
5. THE MINIMUM SEPARATION BETWEEN WATER SERVICE LINES IS 5 FEET.
6. ANY TIE-IN, CROSSING OR LOWERING OF TOWN CRITICAL INFRASTRUCTURE WILL REQUIRE A TIME AND MATERIALS PERMIT (TMU) WITH FEES REQUIRED PRIOR TO CONSTRUCTION.

7. THE EXISTING METER CAPACITY IS TO BE CALCULATED BASED ON THE REQUIRED DEMAND OF ALL THE TOTAL EXISTING AND PROPOSED WATER SUPPLY FIXTURE UNITS AND ANY OTHER ADDITIONAL USAGE, PROVIDED TO THE TOWN OF CASTLE ROCK AT THE TIME OF BUILDING PERMITTING.
8. ANY EXISTING SERVICES NOT USED MUST BE REMOVED AT THE MAIN AND ABANDONED.

associated with the Town's Concept, Preliminary, and Final Development Packages are documented in Chapter 3 of these Criteria.

4.4.1. Design Demands

The tabulation below provides the Average Daily Demand (ADD) rates that shall be applied to compute the demands associated with the various types of land use within the development. Demands shall be computed for intermediate and full build-out phases of the development, and shall be assigned to the appropriate design points within the model to accurately reflect the geographical distribution of land use throughout the site.

Land Use	Typical ADD/Unit	Typical SFEs/Unit
Single Family Residential and Duplexes	400 gpd/dwelling unit	1.00/dwelling unit
Multi-Family Residential (Townhomes and	260 gpd/dwelling unit	0.65/dwelling unit
Retail/Offices	0.2 gpd/SF	0.0005/SF
Hotels/Motels	75 gpd/room	0.19/room
Restaurants	3.0 gpd/SF	0.0075/SF
Car Washes	5,000 gpd/facility	12.5/facility
Industrial / Other Commercial	1,200 gpd/acre	3.0/acre
Institutional	800 gpd/acre	2.0/acre
Rates for Typical Douglas County		
School Sizes:		
Elementary (10 ac, 675 students)	3,375 gpd/school	8.44/school
Middle (25 ac, 850 students)	8,500 gpd/school	21.25/school
High (50 ac, 1,700 students)	17,000 gpd/school	42.50/school
Rates for Alternate School Sizes:		
Elementary – Domestic	5 gpd/student	0.013/student
Middle – Domestic	10 gpd/student	0.025/student
High – Domestic	10 gpd/student	0.025/student
Elementary Schools – Irrigation	3,000 gpd/total acreage	7.5/total acreage
Middle Schools – Irrigation	5,250 gpd/total acreage	13.1/total acreage
High Schools – Irrigation	4,400 gpd/total acreage	11.0/total acreage
Churches	600 gpd/church	1.5/church
Gym/Fitness Center	Approx. Proposed ADD = 600 gpd E/SF	
Irrigated Land (Sports Fields and School Sites Excluded)	3,750 gpd/ irrigated acre	9.38/acre

4.4.1.1 Water System Average Daily Demands (ADD)

Notes:

- (1) All tabulated ADD rates include irrigation demands with the exception of multi-family residential, car washes, gyms/fitness centers, and schools.
- (2) Irrigation demands shall be in accordance with the Irrigation Plan approved by Castle Rock Water and hours of irrigation shall comply with the latest edition of the Town's Water Use Management Plan. Irrigation demands in this Table are for planning purposes only. Peak irrigation rates for 'Irrigated Land' and 'School Irrigation' categories tabulated above shall be in accordance with the approved Irrigation Plan, and shall not be based on the peaking factors identified in the section below.
- (3) The "Irrigated Land" rate identified above may be higher for active sports fields (e.g., irrigated fields within parks) as determined by the approved Irrigation Plan.
- (4) School sizes in this table are for planning purposes, per the Douglas County School District. Regarding design flows for schools, planned school sizes shall be confirmed with the Douglas County School District and documented in the Utility Report. The tabulated rates shown above shall be applied as appropriate, depending on the information provided by the School District.
- (5) Reasonable engineering judgment shall be used in determining the estimated ADD for commercial, industrial and institutional sites. Individual evaluation and justification, including fixture unit counts, shall be provided when the proposed use is not specifically represented in the table above, or when the proposed demands will be inconsistent with the tabulated values. For example, each car wash's unique operating plan, including recycling of wash water, shall be considered. Justification of all non-residential demands shall be addressed in the Utility Report.

4.4.1.2 Demand Factors

Once the ADD has been computed in accordance with the demand rates tabulated above, Maximum Day Demands (MDD) and Peak Hour Demands (PHD) shall be computed by multiplying the ADD by the following factors:

Maximum Day Demand/Average Day Demand:	2.5	MDD/ADD
Peak Hour Demand/Average Day Demand:	5.5	PHD/ADD

Peaking factors are not applied to
Peak Hour Demand are calculated using

Max Daily Demand = 600 gpd x 2.5 = 1,500 gpd
Peak Hour Demand = 600 gpd x 5.5 = 3,300 gpd

factors. If the site's irrigation demands are calculated using the table in section 4.4.1, the irrigation ADD is added to the calculated MDD and PHD, computed as follows:

MDD = Domestic ADD X Maximum Day Factor + Irrigation ADD
PHD = Domestic ADD X Peak Hour Factor + Irrigation ADD

4.4.1.3 Fire Flows

Fire flows shall conform to the latest edition of the International Fire Code (IFC) and shall be confirmed by the Town's Fire Department, as described in section 3.6.XII of

this criteria. A 50 percent reduction in required fire flow may be allowed for approved residential sprinkler systems, and a 75 percent reduction may be allowed for approved commercial sprinkler systems at the discretion of the Town of Castle Rock Fire Chief. The minimum fire flow for the water distribution system is 1,500 gpm, unless otherwise approved by the Fire Department.

Building S.F. = 11,092 S.F.
(1 floor with sprinklers)

TABLE B105.1(2) REFERENCE TABLE FOR TABLES B105.1(1) AND B105.2



FIRE-FLOW CALCULATION AREA (square feet)					FIRE-FLOW (gallons per minute) ^b	FLOW DURATION (hours)
Type IA and IB ^a	Type IIA and IIIA ^a	Type IV and V-A ^a	Type IIB and IIIB ^a	Type V-B ^a		
0-22,700	0-12,700	0-8,200	0-5,900	0-3,600	1,500	2
22,701-30,200	12,701-17,000	8,201-10,900	5,901-7,900	3,601-4,800	1,750	
30,201-38,700	17,001-21,800	10,901-12,900	7,901-9,800	4,801-6,200	2,000	
38,701-48,300	21,801-24,200	12,901-17,400	9,801-12,600	6,201-7,700	2,250	
48,301-59,000	24,201-33,200	17,401-21,300	12,601-15,400	7,701-9,400	2,500	
59,001-70,900	33,201-39,700	21,301-25,500	15,401-18,400	9,401-11,300	2,750	3
70,901-83,700	39,701-47,100	25,501-30,100	18,401-21,800	11,301-13,400	3,000	
83,701-97,700	47,101-54,900	30,101-35,200	21,801-25,900	13,401-15,600	3,250	
97,701-112,700	54,901-63,400	35,201-40,600	25,901-29,300	15,601-18,000	3,500	
112,701-128,700	63,401-72,400	40,601-46,400	29,301-33,500	18,001-20,600	3,750	

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Based on the computed storage volume, Castle Rock Water will evaluate whether the development is compatible with the location and magnitude of the existing and master-planned storage volume within the pressure zone, or if additional storage must be considered to provide the appropriate level of service to the development. Due to hydraulic constraints and limitations within the service area, fire storage within the pressure zone may need to be duplicated as determined by Castle Rock Water.

4.4.3 Minimum Hydraulic Performance Criteria

All mains shall be sized to provide for domestic, irrigation and fire protection demands to all points in the development without violating the pressure, velocity, and headloss criteria set forth herein. The proposed network shall be evaluated by the engineer under the hydraulic conditions stipulated in the following table using an approved hydraulic network analysis model. In some cases, the Town may require the oversizing of certain mains based on required master-planned sizes associated with the overall Town system. Recovery costs of such oversizing shall be in accordance with the Town Municipal Code, and pursuant to applicable executed agreements.

Hydraulic Condition	Pressure (psi)
Minimum static pressure with zero demand (20% tank stage)	43
Maximum static pressure with zero demand (100% tank stage)	125
Minimum dynamic pressures (based on 20% tank stage):	
Max Daily Demand + fire flow (see note below)	20
Peak Hour Demand (see note below)	35

Note: Pressure and Velocity criteria listed above for the MDD plus fire flow and PHD conditions shall be satisfied with one critical distribution loop out of service.

4.4.3.1 Operating Pressures and Pressure Zone Characteristics

The required operating pressures listed above apply to pressures within the mains, and shall be satisfied in accordance with the general pressure zone elevation data tabulated below (Vertical Datum NAVD 88).

Pressure Zone Designation	Max HGL Elev.	Top Elev. Served	Low Elev. Served
1 (Orange)	6192	6072	5915
2 (Yellow)	6352	6230	6065
3a (Blue-east)	6508	6389	6221
3b (Blue-west)	6523	6399	6236
4 (Purple)	6594	6474	6308
5 (Red)	6680	6557	6393
6 (Green)	6824	6700	6537

Notes: (1) Elevations shown above are general guidelines. Engineer shall contact Castle Rock Water to confirm the appropriate numbers for a specific site. (2) Pressure evaluations in areas served solely by PRVs shall be based on the PRV settings, as provided by Castle Rock Water.

4.4.3.2 Assumed Pressures at Existing System Connections

Primary distribution and transmission mains between the supplying storage tank and

the development may be required at the discretion of Castle Rock Water to account for the headloss between the supply source and the location of the development under MDD and PHD conditions. In lieu of this information, Castle Rock Water may provide the engineer with assumed Master Plan pressures or HGL elevations at the proposed connections as the necessary boundary conditions for the required hydraulic models.

4.4.3.3 Maximum Velocities and Headlosses

Maximum velocity with Peak Hour Demands:	10 fps
Maximum velocity with Max Day Demands:	5 fps
Maximum velocity with Max Day + Fire Flow Demands	15 fps
Maximum headloss through 16" (transmission):	2.0 ft/1000 ft
Maximum headloss through 20" (transmission):	1.5 ft/1000 ft
Maximum headloss 24" and larger (transmission):	1.0 ft/1000 ft
Hazen Williams 'C' factor (PVC & DIP):	120

<u>Distribution Main Sizing</u>	<u>Minimum Standard</u>
Minimum pipe size (see note below):	8 inches
Minimum pipe size in an arterial street:	12 inches
<u>Valve Spacing</u>	<u>Minimum Standard</u>
Largest spacing on transmission mains:	1200 feet
Largest spacing on distribution mains:	600 feet

Note: 'C' factor includes allowances for minor losses and pipe aging.

4.4.4 General Water System Layout Criteria

4.4.4.1 Location

All water mains and appurtenances shall be installed in public right-of-way, dedicated utility easements and Town property. Water mains shall not be installed parallel to and directly below, any concrete such as sidewalks, trails, curbs, or gutters, and no water manholes or appurtenances shall be located in multi-use trails and sidewalks. Mains shall be located in accordance with the Standard Details available on the Town's website, unless otherwise approved by variance from Castle Rock Water. In public streets, water mains shall be located near the center of the north and east lane wherever possible, or seven feet minimum from edge of main to

4.4.1.1 Wastewater System Average Daily Flow (ADF)

Land Use	Typical ADF/Unit	Typical SFEs/Unit
Single Family Residential and Duplexes	200 gpd/dwelling unit	1.00/dwelling unit
Multi-Family Residential (Townhomes and Condos)	130 gpd/dwelling unit	0.65/dwelling unit
Retail/Offices	0.1 gpd/SF	0.0005/SF
Hotels/Motels	75 gpd/room	0.375/room
Restaurants	1.5 gpd/SF	0.0075/SF
Car Washes	5,000 gpd/acre	25.0/acre
Industrial/Other Commercial	600 gpd/acre	3.0/acre
Institutional	400 gpd/acre	2.0/acre
Churches	300 gpd/church	1.5/church
Gym/Fitness Center	Approx. Proposed ADF = 300 gpd SFE/SF	
Irrigated Land	0 gpd/acre	0.0/acre
<u>Rates for Typical Douglas County School Sizes:</u>		
Elementary (10 ac, 675 Students)	3,375 gpd/school	16.88/school
Middle (25 ac, 850 students)	8,500 gpd/school	42.50/school
High (50 ac, 1700 students)	17,000 gpd/school	85.00/school
<u>Rates for Alternate School Sizes:</u>		
Elementary	5 gpd/student	0.025/student
Middle	10 gpd/student	0.050/student
High	10 gpd/student	0.050/student

The demands noted in the table above represent the “winter time” or “dry weather” Average Daily Flow (ADF) with no allowance for infiltration and inflow (I/I). I/I shall be accounted for in accordance with the Peak Design Flow computation discussed in the sections to follow.

Regarding design flows for schools, planned school sizes shall be confirmed with the Douglas County School District and documented in the Utility Report. The tabulated rates

shown above shall be applied as appropriate, depending on the information provided by the School District. Reasonable engineering judgment shall be used in determining the estimated ADF for commercial, industrial and institutional sites. Individual evaluation and justification shall be provided when the proposed use is not specifically represented in the table above, or when the proposed flows will be inconsistent with the tabulated values. Justification of all non-residential flows shall be addressed in the Utility Report.

4.4.1.2 Peaking Factor

The portion of the Peak Design Flow calculation exclusive of I/I shall be determined by multiplying the ADF in the preceding section by a peaking factor (PF). The PF is a function of ADF, and shall be computed as follows: $PF = 3.65 \times [(ADF)^{-0.168}]$.

The maximum PF shall be 5.0, and ADF shall be expressed in units of million gallons per day (MGD).

$$\text{Peaking Factor} = 3.65 \times [(0.00003 \text{ MGD})^{-0.168}] = 14.2$$
$$\text{PF} = 5.0 \text{ max}$$

4.4.1.3 Peak Design Flow Rate

The Peak Design Flow (PDF) shall be computed at all pertinent design points throughout the system, and used to size the corresponding wastewater facilities. The PDF shall be determined by multiplying the ADF by the calculated peaking factor (PF), plus an I/I allowance equal to ten percent of the ADF as expressed by the following formula:

$$\text{PDF} = \text{peak base flow} + \text{I/I allowance} = (\text{ADF} \times \text{PF}) + (0.1 \times \text{ADF})$$

Once computed, the PDF shall be used to determine the sizes of the facilities in accordance with the criteria established below.

$$\text{Peak Daily Flow} = (300 \text{ gpd} \times 5) = (0.1 \times 300 \text{ gpd}) = 1,530 \text{ gpd}$$
$$\text{PDF} = 1,530 \text{ gpd}$$

4.4.2 Hydraulic Design

Wastewater systems shall be designed to provide a minimum velocity of two feet per second at the Peak Design Flow. In situations where the minimum velocity of 2.0 feet per second cannot be met, an explanation and justification shall be included in the Final Utility Report. The maximum velocity shall not exceed ten feet per second. Drop manholes shall be provided to break steep slopes to limit the velocities in conformance with this criterion. Where drop manholes are impractical for velocity reduction, the sewer must be approved by variance and shall be of PVC or other abrasion resistant material approved by Castle Rock Water.

Where actual flow will be much below normal for several years, the minimum velocity shall be achieved by suitable grades at the partial design flow rate. Care shall be taken to design invert elevations at manholes in such a manner that the energy gradient is consistently falling in the direction of the flow. Sanitary sewers shall be sized to convey the Peak Design Flow while flowing no more than 75 percent of the maximum pipe flow capacity. Computation of velocity of flow shall be based on a coefficient of roughness "n" in the Manning formula of 0.011 for PVC. At all junctions where a smaller diameter main discharges into a larger one, the invert of the larger sanitary sewer shall be lowered so that the energy gradients of the sewers at the junction are at the same level. Generally, this condition will be met by placing the 0.80 depth point of both sewers at



Commercial Business Building Permit

Grease Interceptor Requirement

All Commercial Business Building Permits for new business and construction of new buildings, remodels, or tenant improvement permits will be required to comply with Town Code 13.05, Grease Interceptors Required. Applicant will be required to submit an Industry Information Questionnaire through the Plum Creek Water Reclamation Authority (PCWRA) to verify if the business will require an oil/grease interceptor. Please go to PCWRA website at:

<https://www.pcwracolorado.org/commercial-businesses>

CONSTRUCTION DRAWINGS

Please note PCWRA requires all new commercial buildings to provide a stub-out for a separate waste line for future interceptor installation, if not proposed with the project. All drainage fixtures and grease waste lines that are tied to the oil/grease must be shown in the construction drawings as well as the location of the oil/grease interceptor.

ACKNOWLEDGEMENT

The applicant must sign and date this form to acknowledge that the applicant has read and understands the Town's Code requirement regarding Grease Interceptors, PCWRA's requirement to complete an Industry Information Questionnaire and Food Service Application, and the Town's requirement that a letter from PCWRA must be provided with the Town of Castle Rock Building Permit. This signed form is required to be included in the Utility Report with the Predevelopment Plan (PDP), Site Development Plan (SDP) and Construction Document (CD) submittals.

Castle Oaks Church - Site Development Plan

Name of Project

826 Park Street, Castle Rock, CO 80109

Address of property, if known

Signature

12/4/2025

Date